

# Analysis of the Effect and Influencing Factors of Rural Domestic Sewage Treatment Based on A<sup>2</sup>O-MBBR Integrated Process

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**Abstract** [Objectives] This study was conducted to solve the prominent problems in the treatment of domestic sewage in southern rural areas of China. [Methods] An integrated process treatment mode of anaerobic/anoxic/aerobic moving bed biofilm reactor (A<sup>2</sup>O-MBBR) was proposed to analyze and study its operating effect and influencing factors. [Results] The A<sup>2</sup>O-MBBR mode had good COD removal efficiency and nitrogen and phosphorus removal performance, and the water quality index of the effluent met the Class A standard of GB181918–2002. This mode is suitable for treating rural domestic sewage, and has high treatment effects in different operating periods. In spring, the average removal rates of COD, NH<sub>4</sub><sup>+</sup>-N, TN, TP and SS reached (83.53 ± 2.15)%, (89.44 ± 4.97)%, (67.36 ± 18.53)%, (88.22 ± 11.21)% and (91.73 ± 2.25)%, respectively; In the autumn period, the average removal rates of COD, NH<sub>4</sub><sup>+</sup>-N, TN, TP and SS were (83.49 ± 2.64)%, (89.26 ± 9.19)%, (66.05 ± 17.00)%, (87.48 ± 9.68)%, and (91.13 ± 2.35)%. [Conclusions] This study provides theoretical reference and technical support for the popularization and application of A<sup>2</sup>O-MBBR integrated process.

**Key words** A<sup>2</sup>O-MBBR process; Rural domestic sewage; Nitrogen and phosphorus removal

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With the acceleration of rural revitalization in China, domestic water consumption in rural areas has risen sharply, and domestic sewage output and emissions have increased significantly. In recent years, the CPC Central Committee and the State Council have attached great importance to the treatment of rural domestic sewage in China, and successively promulgated a series of rural environmental protection policies and technical documents, such as *Guiding Opinions on Promoting the Treatment of Rural Domestic Sewage* (Zhong Nong Fa [2019] No. 14), *Guiding Opinions on Promoting the Treatment of Black and Odorous Water Bodies in Rural Areas* (Huanban Turang Han [2019] No. 48) and *Five-year Action Plan for Improving and Upgrading Rural Living Environment* (2021–2025). It can be seen that seeking more efficient and economical domestic sewage treatment techniques is an important measure to deal with the increasingly severe water environment situation in rural areas of China and an inevitable demand for the revitalization of rural industries.

At present, the treatment methods of rural domestic sewage in China include: constructed wetland<sup>[1]</sup>, biological filter<sup>[2]</sup>, biological contact oxidation tank<sup>[3]</sup>, sequencing batch bioreactor (SBR)<sup>[4]</sup>, membrane bioreactor (MBR)<sup>[5]</sup>, moving bed biofilm reactor (MBBR)<sup>[6]</sup> and other processes and facilities. Each process treatment technique has its own characteristics and applicability. A municipal sewage treatment plant in Huai'an combines

A<sup>2</sup>O (anaerobic-anoxic-oxic) and MBBR (moving bed biofilm reactor) to treat domestic sewage with an improved pollutant removal rate<sup>[7]</sup>. The A<sup>2</sup>O process plays a main role in removing organic pollutants, suspended solids and nitrogen and phosphorus, and the treatment effect is good<sup>[8]</sup>. MBBR is a sewage treatment technique based on biofilm process. It provides a suitable environment for the growth of microorganisms and increases the amount of active microorganisms in the system by adding suspended filler with density similar to that of water into the system, thus making the system more resistant to impact load<sup>[9]</sup>. The MBBR process can effectively make up for the shortcomings of traditional activated sludge process and fixed bed biofilm technique<sup>[10]</sup>. It not only has good process flexibility, but also is widely used in the upgrading and efficiency improvement of sewage treatment plants<sup>[11–13]</sup>, so it has good engineering application prospects<sup>[14]</sup>. Therefore, the development of A<sup>2</sup>O-MBBR integrated process for domestic sewage treatment has better engineering guiding significance in the practical application of rural domestic sewage treatment.

In this study, the A<sup>2</sup>O-MBBR integrated process treatment mode was analyzed for its characteristics including effluent stability, stable compliance rate and pollutant removal rate of rural domestic sewage, and its design and operation problems were analyzed, hoping to provide reference and technical support for the popularization and application of A<sup>2</sup>O-MBBR integrated process treatment.

## Technological Process

### Process overview

In this study, the A<sup>2</sup>O-MBBR integrated process treatment facilities, with a scale of 200 m<sup>3</sup>/d, included four tanks in each reaction zone, namely anaerobic tank, anoxic tank, aerobic tank

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and sedimentation tank, and the effective volumes in various zones were 19.5, 18.5, 25.5 and 13.5 m<sup>3</sup> respectively. In the experimental study, the anaerobic tank, anoxic tank and aerobic tank of the A<sup>2</sup>O unit were all filled with elastic filler, and the filling rate was about 55%–70%. The filler was a flat cylindrical polyethylene suspended carrier with a diameter of 25 mm, a height of 10 mm, a density of 0.96–0.98 g/cm<sup>3</sup> and a specific surface area of 600–700 m<sup>2</sup>/m<sup>3</sup>, which met the industry standard requirements of *High Density Polyethylene Suspended Carrier for Water*

*Treatment* (CJ/T461-2014)<sup>[15]</sup>. Interception screens were arranged at the junction of anoxic tank and aerobic tank, and at the junction of aerobic tank and sedimentation tank. A secondary sedimentation tank was arranged behind the aerobic tank, and the sludge liquid in the secondary sedimentation tank flow back to the anaerobic tank through an air lift mud pump to reduce sludge discharge.

### Process flow

The main flow diagram of A<sup>2</sup>O-MBBR integrated process is shown in Fig. 1.

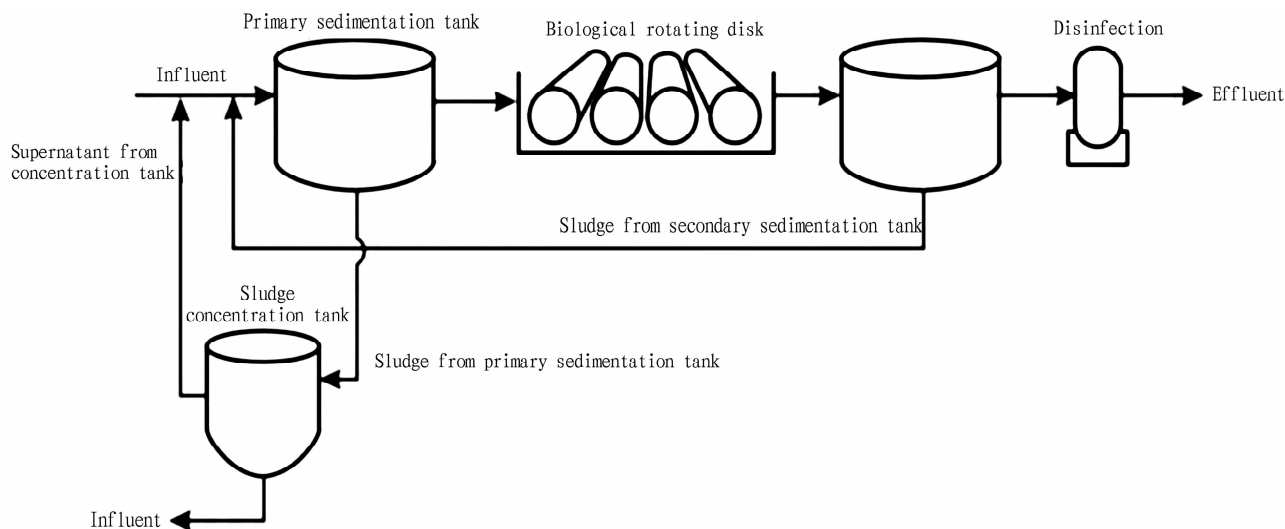


Fig. 1 Schematic diagram of the main process flow of A<sup>2</sup>O-MBBR integrated process treatment

## Materials and Methods

### Research object

In this study, the domestic sewage facilities in Heping Village, Ansha Town, Changsha County, Hunan Province were taken as the object, and the domestic sewage of the A<sup>2</sup>O-MBBR integrated process treatment facilities was sampled on the spot, and divided into two phases: winter and summer. The sampling date in winter is from November to December, 2022, and there was continuous rainfall during the sampling period. The sampling time in summer was from June to August, 2023, and there was no rain in the first 3 d before sampling.

### Sample collection

The number of samples taken at each station in the experimental area was three, and no parallel samples were set, but quality control samples were set. For each treatment facility, 3–5 bottles of water samples were collected from the water collecting well, the A<sup>2</sup>O secondary sedimentation tank and the water discharging

well in turn. Each bottle of water sample was 550 ml, and the collected samples served as the influent and effluent of A<sup>2</sup>O, respectively. The water sample collection process was strictly in accordance with the requirements. The collected samples were packed in sterile plastic bags and immediately stored at 4 °C. The test indexes included COD, NH<sub>3</sub>-N, TN, TP and SS, and the reagents were analytically pure according to the national standard method<sup>[16]</sup>. The analysis and test time was: NH<sub>3</sub>-N and TN < 24 h, TP < 48 h, COD and SS < 72 h.

### Discharge standards

The water quality indexes of COD, NH<sub>3</sub>-N, TN, TP and SS in the influent of the reactor and the effluent of the sedimentation tank were measured in spring and autumn in various stages of the experimental area. The quality indexes of effluent reached the Class A standard of *Discharge Standard of Pollutants for Municipal Wastewater Treatment Plant* (GB181918-2002). An A<sup>2</sup>O influent and effluent quality standard table was designed for the process (Table 1).

Table 1 A<sup>2</sup>O influent and effluent quality standards

Water quality indexes	COD <sub>Cr</sub>	NH <sub>3</sub> -N	TN	TP	SS	PH
Influent	300	40	50	5	200	6–9
Effluent	≤50	≤5	≤15	≤0.5	≤10	6–9

### Statistical analysis of data

According to the measured data, the software Origin (2022)

and SPSS were used for clustering analysis.

## Results and Analysis

### Analysis on operation effect of A<sup>2</sup>O-MBBR

**Analysis on COD removal performance** During the experiment,

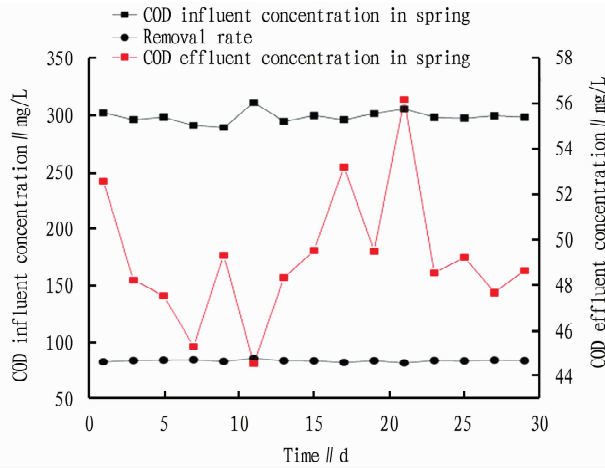
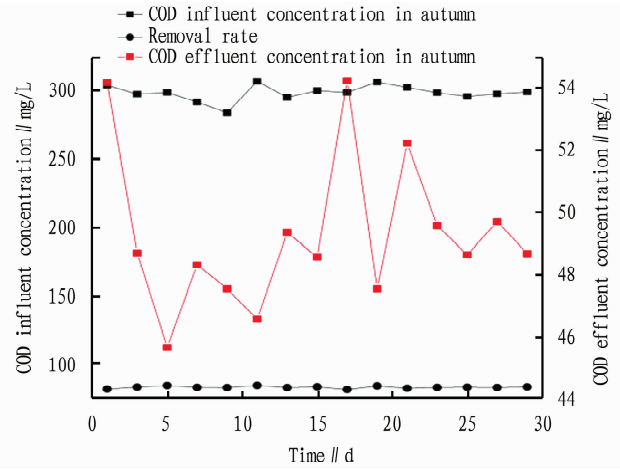


Fig. 2 COD removal effect of A<sup>2</sup>O-MBBR integrated process treatment

It could be seen that in spring, the COD concentrations in the influent of reactor and effluent of sedimentation tank were  $(298.99 \pm 12.22)$  and  $(49.24 \pm 6.97)$  mg/L, respectively, and the average COD removal rate reached  $(83.53 \pm 2.15)\%$ . In autumn, the COD concentrations in the influent of reactor and effluent of sedimentation tank were  $(298.60 \pm 15.58)$  and  $(49.29 \pm 8.59)$  mg/L, respectively, and the average COD removal rate

reached  $(83.49 \pm 2.64)\%$ . The A<sup>2</sup>O-MBBR integrated process had strong COD removal performance.



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**Analysis on NH<sub>3</sub>-N removal performance** During the experiment, the NH<sub>3</sub>-N concentrations in the influent and effluent at various stages of A<sup>2</sup>O-MBBR integrated process and its removal effect are shown in Fig. 3.

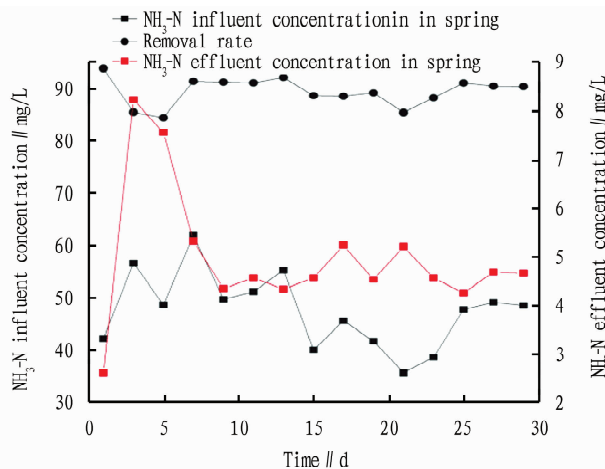
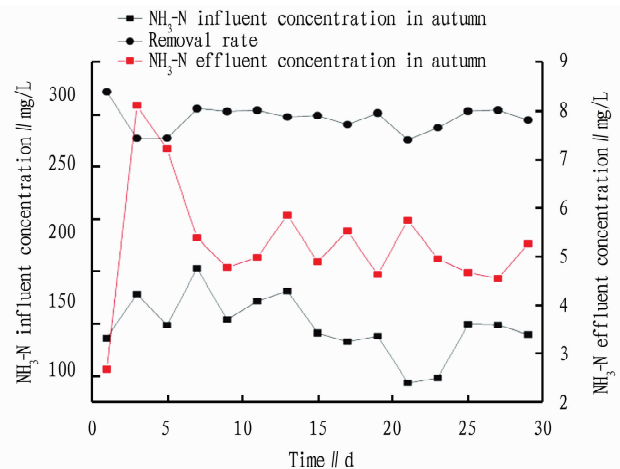


Fig. 3 NH<sub>3</sub>-N removal effect of A<sup>2</sup>O-MBBR integrated process treatment

It could be seen that in spring, the NH<sub>3</sub>-N concentrations in the influent of reactor and effluent of sedimentation tank were  $(47.54 \pm 9.13)$  and  $(4.98 \pm 3.23)$  mg/L, respectively, and the average NH<sub>3</sub>-N removal rate reached  $(89.44 \pm 4.97)\%$ . In autumn, the NH<sub>3</sub>-N concentrations in the influent of reactor and effluent of sedimentation tank were  $(49.49 \pm 17.62)$  and  $(5.28 \pm 3.43)$  mg/L, respectively, and the average NH<sub>3</sub>-N removal rate reached  $(89.26 \pm 9.19)\%$ . The A<sup>2</sup>O-MBBR integrated process had strong NH<sub>3</sub>-N removal performance.

**Analysis on TN removal performance** During the experiment, the TN concentrations in the influent and effluent at various stages

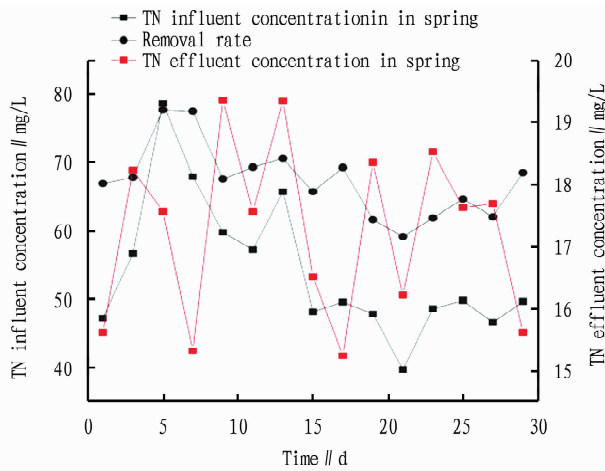


of A<sup>2</sup>O-MBBR integrated process and its removal effect are shown in Fig. 4.

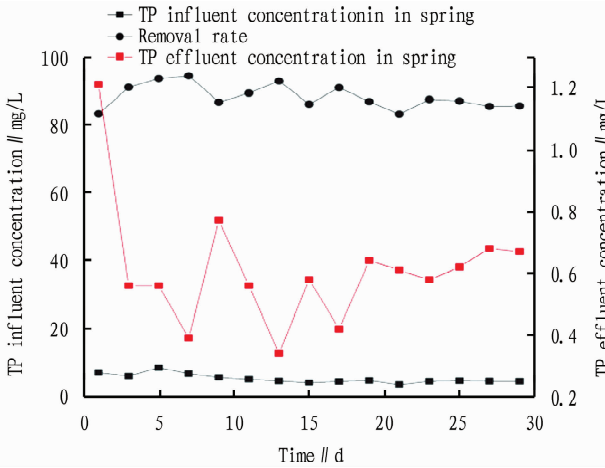
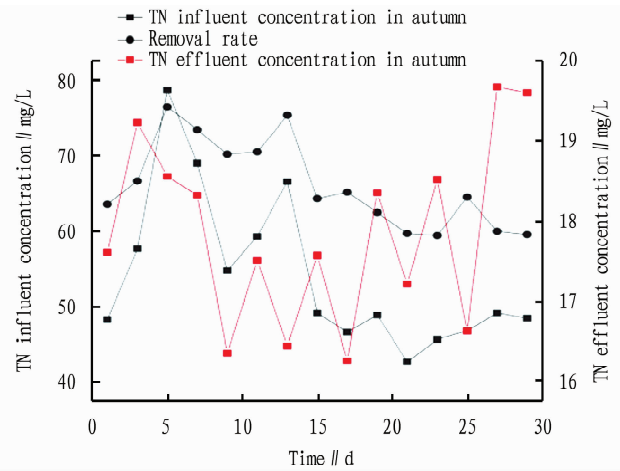
It could be seen that in spring, the concentrations of TN in the influent of reactor and effluent of sedimentation tank were  $(54.22 \pm 24.46)$  and  $(17.25 \pm 4.11)$  mg/L, respectively, and the average TN removal rate reached  $(67.36 \pm 18.53)\%$ . In autumn, the concentrations of TN in the influent of reactor and effluent of sedimentation tank were  $(54.23 \pm 21.26)$  and  $(17.86 \pm 3.43)$  mg/L, respectively, and the average TN removal rate reached  $(66.05 \pm 17.00)\%$ . The removal effect of TN by A<sup>2</sup>O-MBBR integrated process was remarkable.

**Analysis on TP removal performance** During the experiment, the TP concentrations in the influent and effluent at various stages

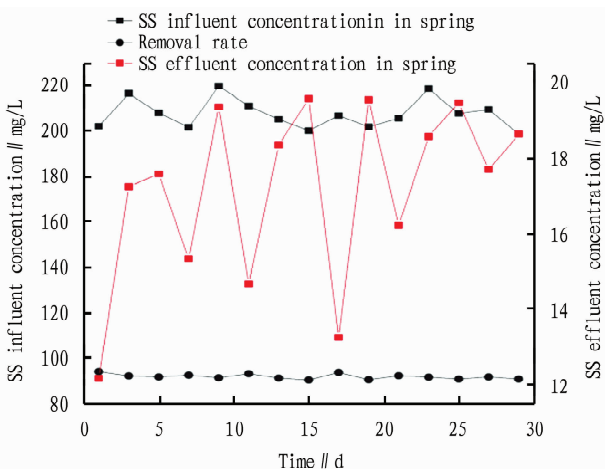
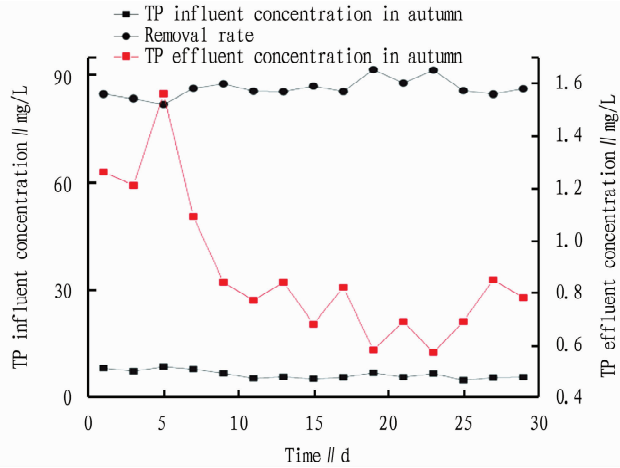
of A<sup>2</sup>O-MBBR integrated process and its removal effect are shown in Fig. 5.



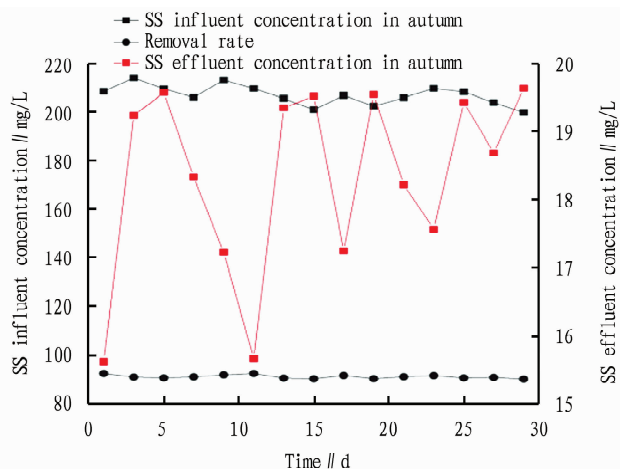
**Fig. 4** TN removal effect of A<sup>2</sup>O-MBBR integrated process treatment



**Fig. 5** TP removal effect of A<sup>2</sup>O-MBBR integrated process treatment



**Fig. 6** SS removal effect of A<sup>2</sup>O-MBBR integrated process



It could be seen that in spring, the TP concentrations in the influent of reactor and effluent of sedimentation tank were  $(5.38 \pm 3.2)$  and  $(0.61 \pm 0.28)$  mg/L, respectively, and the

average TP removal rate reached  $(88.22 \pm 11.21)\%$ . In autumn, the concentrations of TP in the influent of reactor and effluent of sedimentation tank were  $(6.37 \pm 3.83)$  and  $(0.88 \pm 0.38)$  mg/L,

respectively, and the average TP removal rate reached  $(87.48 \pm 9.68)\%$ . The integrated A<sup>2</sup>O-MBBR process had a good TP removal effect.

**Analysis on SS removal performance** During the experiment, the SS concentrations in the influent and effluent at various stages of A<sup>2</sup>O-MBBR integrated process and its removal effect are shown in Fig. 6.

It could be seen that in spring, the SS concentrations in the influent of the reactor and the effluent of the sedimentation tank were  $(207.62 \pm 11.21)$  and  $(17.17 \pm 5.01)$  mg/L, respectively, and the average SS removal rate reached  $(91.73 \pm 3.75)\%$ . In autumn, the concentrations of SS in the influent of reactor and effluent of sedimentation tank were  $(206.63 \pm 13.20)$  and  $(18.31 \pm 4.00)$  mg/L, respectively, and the average removal rate of SS reached  $(91.13 \pm 2.35)\%$ . The removal effect of SS by A<sup>2</sup>O-MBBR integrated process was significantly improved.

**Analysis on influencing factors of A<sup>2</sup>O-MBBR operation**

**Influencing factors of COD removal** As shown in Fig. 2, there was no significant difference in COD removal rate between various operation stages of A<sup>2</sup>O-MBBR integrated process reactor ( $P=0.445$ ). Main influencing factors: First, the removal rate of COD was related to the concentration of DO in the aerobic tank, and the COD removal effect did not change significantly in each operation stage of the integrated process reactor when the concentration of DO changed slightly. Second, the removal rate of COD had little correlation with the influent quality concentration. When the water quality concentration fluctuated greatly, the integrated process reactor still maintained a stable COD removal effect at each operation stage, showing excellent tolerance. Third, the removal rate of COD was not related to the influent COD/N ratio, and the removal effect of COD in each operation stage of the integrated process reactor was not significantly influenced. It is consistent with the research results of Machat *et al.*<sup>[17]</sup>. It could be seen that the removal of COD in domestic sewage must be carried out by anaerobic reaction, and under the disturbance of DO, influent COD/N ratio and volume loading, the integrated process reactor could maintain a stable COD removal effect in each operation stage.

**Influencing factors of NH<sub>3</sub>-N removal** As can be seen from Fig. 3, the NH<sub>3</sub>-N removal effect of A<sup>2</sup>O-MBBR integrated process reactor in various operation stages was significantly improved ( $P=0.292$ ). Main influencing factors: the removal rate of NH<sub>3</sub>-N was related to DO concentration in the aerobic tank. When  $DO \leq 2.0$  mg/L, the demand for nitrification reaction was slow; when  $2.5 \leq DO \leq 3.5$  mg/L, the removal rate of NH<sub>3</sub>-N reached  $(89.44 \pm 4.97)\%$ ; and when  $DO \geq 3.5$  mg/L, no significant improvement was shown in the removal effect of NH<sub>3</sub>-N. The conclusion is similar to that of Zheng *et al.*<sup>[18]</sup> and Wang *et al.*<sup>[19]</sup>. It could be seen that DO formed aerobic zones and anaerobic zones in different thickness periods of biofilm, which promoted nitrification and anaerobic ammonification and improved NH<sub>3</sub>-N removal efficiency.

**Influencing factors of TN removal** As can be seen from Fig. 4, the TN removal rate of A<sup>2</sup>O-MBBR integrated process reactor was

improved significantly in various operation stages ( $P < 0.023$ ). Main influencing factors: First, the removal rate of TN was closely related to influent water quality C/N. If the influent water quality C/N was low, the removal rate of TN would be low. Second, the removal rate of TN was related to DO concentration. Because of the difference of DO concentration inside and outside the biofilm, aerobic bacteria such as nitrifying bacteria grew on the surface of the biofilm, anaerobic bacteria such as denitrifying bacteria grew inside the biofilm, and the difference of bacteria inside and outside the biofilm affected the removal effect of TN. Third, the removal rate of TN was affected by diversified factors such as reflux ratio and biomass. It is similar to the research results of Wang *et al.*<sup>[20]</sup> and Yang *et al.*<sup>[21]</sup>. It could be seen that the removal rate of TN was affected by many factors, such as influent water quality C/N, DO, reflux ratio and biomass. In actual operation, the reflux ratio of nitrification liquid in A<sup>2</sup>O-MBBR integrated process should not be too high, and the influent C/N should not be too low.

**Influencing factors of TP removal** As shown in Fig. 5, the effluent TP concentration in various operation stages of A<sup>2</sup>O-MBBR integrated process reactor were relatively high ( $P=0.112$ ). Main influencing factors: First, the volume space of various compartments in the equipment was limited, and there was a backmixing phenomenon between nitrate nitrogen and dissolved oxygen carried by reflux and adjacent aerobic compartments, which affected the anaerobic environmental conditions in the equipment and led to insufficient biological phosphorus removal conditions. Second, the phosphorus removal function of A<sup>2</sup>O was mainly realized by discharging sludge. If the C/N of rural domestic sewage was low and the output of activated sludge was low, the biological phosphorus removal capacity of MBBR would be limited. Third, the biological removal of phosphorus of MBBR was related to microbial biomass, that is, the material exchange between biofilm and substrate was related to microbial assimilation. It is consistent with the research results of Zhang<sup>[22]</sup>. Therefore, in the actual operation, the space in various compartments of the equipment of the A<sup>2</sup>O-MBBR integrated process reactor is appropriate, and it is suggested to add an appropriate amount of microorganisms and replace the packing matrix in the equipment regularly during the process treatment to improve the removal rate of TP.

**Influencing factors of SS removal** It could be seen from Fig. 6 that under the conditions of anaerobic, anoxic and aerobic alternate operation, the phosphorus content in SS was relatively high, and the SVI value was generally less than 100 ( $P=0.068$ ). Main influencing factors: First, the removal rate of SS was related to the P content in the influent. When the P/BOD value was high, the filamentous microorganisms in the reaction tank failed to proliferate in large quantities, making it difficult for the filamentous expansion of sludge to occur, and there was a certain limit to the growth of SS, which made it difficult to improve the SS removal rate. Second, the removal rate of SS was related to DO concentration. When the DO concentration of the treated water entering the sedimentation tank was low, it would lead to an anaerobic state and phosphorus release from sludge, and the SS removal rate would decrease. It could be seen that in actual operation, the

influent tank of A<sup>2</sup>O-MBBR integrated process must maintain a certain concentration of DO and the concentration should not be too high, and the residence time should be reduced, so as to avoid the phenomena such as anaerobic state, phosphorus release from sludge and interference of circulating mixed liquid with the anoxic reactor.

## Conclusions and Discussion

(1) The A<sup>2</sup>O-MBBR integrated process was adopted to treat rural domestic sewage. The pollutant removal effect was stable, and the water quality indexes of the effluent reached the Class A standard of GB181918-2002. In spring, the average removal rates of COD, NH<sub>4</sub><sup>+</sup>-N, TN, TP and SS were (83.53 ± 2.15)% , (89.44 ± 4.97)% , (67.36 ± 18.53)% , (88.22 ± 11.21)% and (91.73 ± 2.25)% , respectively. In autumn, the average removal rates of COD, NH<sub>4</sub><sup>+</sup>-N, TN, TP and SS were (83.49 ± 2.64)% , (89.26 ± 9.19)% , (66.05 ± 17.00)% , (87.48 ± 9.68)% and (91.13 ± 2.35)% , respectively.

(2) In the mode of A<sup>2</sup>O-MBBR integrated process for treating rural domestic sewage, the removal effect of pollutants was related to the designed hydraulic load of A<sup>2</sup>O-MBBR, outlet position, influent carbon-nitrogen ratio and less sludge discharge.

(3) There is room for improvement in the mode of A<sup>2</sup>O-MBBR integrated process for treating rural domestic sewage. Suggestions: The first is to optimize the sewage reflux ratio of each reaction tank. During the process, there will be a phenomenon of large instantaneous quantity and small instantaneous quantity in each reaction tank, which will affect the duration of microbial residence reaction. The second is to optimize the control of dissolved oxygen, because it is difficult to control dissolved oxygen in the anaerobic tank and anoxic tank, which affects the effect of nitrogen and phosphorus removal in the whole process. The third is to optimize the space of the equipment, such as increasing the unit volume of the reaction tank and optimizing the reactor structure, so as to improve the removal effect of various pollutants and give full play to the functions of A<sup>2</sup>O-MBBR integrated process treatment facilities.

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